

4.3.1 Introduction

The information presented in this sub-basin plan for Hightower Creek is intended to provide the reader with information necessary to understand the physical setting, methodology used, water quantity problems, results, alternatives evaluation, and recommendations. Section 2 of this study describes in greater details the general methodology, including data collection, engineering methods, and regional analysis.

4.3.2 Sub-basin Information

This section outlines information on the Hightower Creek Sub-basin infrastructure, and its ability to meet level of service requirements. The Hightower Creek sub-basin extends from its confluence with Sugar Creek in the north to James Road and Westside Road in the south. The area of the sub-basin is approximately 1.6 sq mi (1,019 acres), which was divided into 14 hydrologic units ranging from 18 to 203 acres in size. The hydrologic unit boundaries and the In-stream PSWMS are shown on **Figure 4.3.1**. The HU delineation along with the areas and the loading node for each HU is shown in **Table 4.3.1**. A considerable portion of the tributary area to Hightower Creek, about 500 acres, is outside the City limits.

The predominant land use in the sub-basin is Medium Density Residential, which accounts for a little over 50 percent of the total land use. The land use categories along with their respective associated area and percentage for all of One Mile Branch sub-basin are shown in **Table 4.3.2**. The predominant soil within the sub-basin is B. **Table 4.3.3** shows the soils breakdown based on HSG. The soil coverage, infiltration and storage capacity was based on the available data from the NRCS Lowndes County soil survey. Detailed discussion on the Soils and Land Use is available in the Methodology Section of the report.



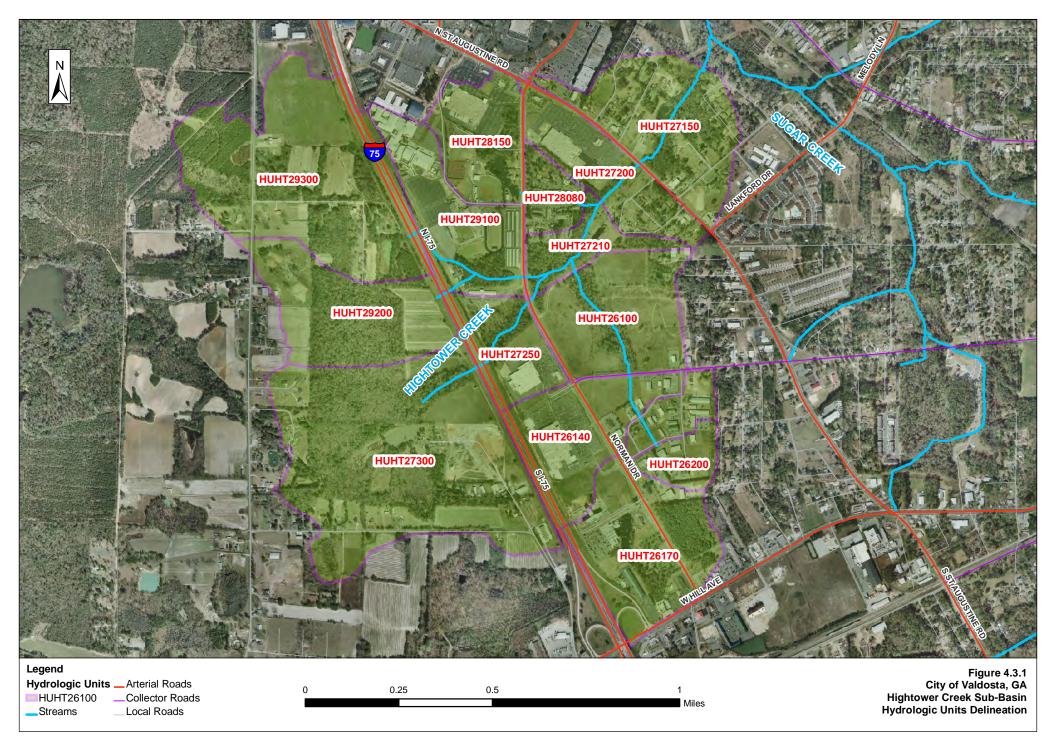




Table 4.3.1. Hydrologic Unit Areas

Hydrologic Unit ID	Area (Acres)	Loading Node			
HUHT26100	73.0	HT32110W			
HUHT26140	63.8	HT32145W			
HUHT26170	100.3	HT32170S			
HUHT26200	17.7	HT32200S			
HUHT27150	81.5	HT30150			
HUHT27200	52.0	HT30200S			
HUHT27210	26.6	HT30210			
HUHT27250	38.4	HT30250S			
HUHT27300	203.0	HT30350AP			
HUHT28080	18.6	HT31100APS			
HUHT28150	46.7	HT31150APS			
HUHT29100	63.2	HT33100S			
HUHT29200	77.1	HT33200S			
HUHT29300	156.7	HT33300S			
Total	1,018.7				

Table 4.3.2. Land Use

Land Use Category	Area (Acres)	Area (Percent)	
Forest, Open & Park	474.4	46.6	
Pasture	0.0	0.0	
Agricultural	23.6	2.3	
Low Density Residential	0.0	0.0	
Medium Density Residential	112.3	11.0	
High Density Residential	0.0	0.0	
Light Industrial, Commercial & Institutional	277.9	27.2	
Heavy Industrial & Roadways	124.3	12.2	
Wetlands	0.0	0.0	
Watercourses & Water bodies	6.3	0.6	
Total	1,018.7	100.0	

Table 4.3.3. Soils

Hydrologic Soil Group	Area (Acres)	Area (Percent)		
A	27.4	2.7		
В	713.3	70.0		
С	132.8	13.0		
D	145.2	14.3		
Total	1,018.7	100.0		



The In-stream PSWMS consists of a main stem channel that at its most downstream section confluences with the Sugar Creek. A schematic showing the model representation (hydraulic network along with nodes) of the sub-basin is presented on **Figure 4.3.2**.1.

4.3.3 Existing Conditions

Some projects in the Hightower Creek sub-basin were proposed in the 1996 MSMP, but none was implemented. However, some culvert upgrades have been implemented and some stormwater pipes have been installed to improve the drainage conditions in the sub-basin.

The Norman Spells Pond just upstream of the Norman Drive has been filled in. A temporary detention area to provide treatment of stormwater runoff coming off the Wal-Mart and neighboring commercial establishments was constructed just upstream of River Street on Hightower Creek.

4.3.4 Water Quantity Problem Areas

1. St. Augustine Road: During field investigation, severe channel undercutting and loss of sand bags downstream of the St. Augustine Road culvert was observed.

4.3.5 Results

The following paragraphs discuss the water quantity model results, the existing level of service in terms of roads flooding, and sediment loads due to erosion.

4.3.5.1 Water Quantity Results

The stages for the 1.2-in, 5-, 25-, 50-, and 100-year 24-hour design storms model runs are presented in **Table 4.3.4**. Road crown elevation, road names, and road classification (local, collector, arterial) are also shown in the table. The roads not meeting the City's defined Level of Service are highlighted in the model result tables. Due to lack of data in terms of finished floor elevations of houses and other structures, available topographic data were utilized to estimate potential flooding of structures for each design storm and tabulated. The model results table indicates the nearest node to the structures flooding location.

4.3.5.2 Total Suspended Solids (TSS) and Channel Bank Erosion Evaluation

Significant sediment loads resulting from erosion of stream banks has been observed in the whole Sugar Creek basin and in One Mile Branch. As per the findings of the Geomorphologic assessment report (Section 3 of this report) of the Sugar Creek, this increase in sediment loads is generated by down cutting of the channel bed (incision), scour of the stream banks or both.



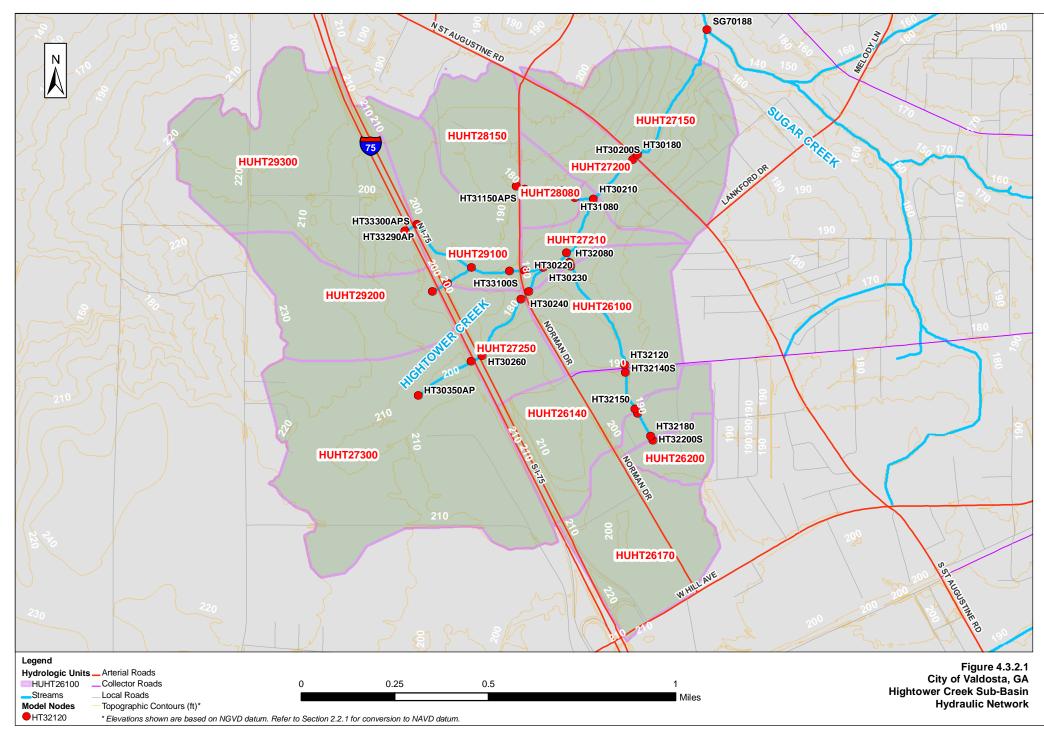


Table 4.3.4. Hightower Creek Existing Condition Model Results

					Design Event				
			Road		Peak Water Surface Elevation (ft-NAVD)				
			Crown	Potential					
		Road	Elevation	Structure					
Node ID	Road Name	Class	(ft-NAVD)	Flooding	1.2 Inch	5 Year	25 Year		100 Year
HT30160					147.7	149.5	150.1	150.3	150.7
HT30170					152.3	153.4	153.8	153.9	154.1
HT30180					156.7	157.7	158.0	158.1	158.3
HT30200S	St. Augustine Road	Arterial	165.6		157.7	162.3	165.3	166.2	166.6
HT30210					160.2	163.1	165.5	166.3	166.8
HT30220					164.3	167.6	168.7	169.0	169.2
HT30230					168.3	170.8	171.1	171.2	171.3
HT30240					170.6	171.5	171.6	171.7	171.7
HT30250S	Norman Drive	Arterial	181.8		172.0	175.5	177.4	178.0	178.5
HT30260					191.5	192.5	192.8	192.9	192.9
HT30300S	I-75	Arterial	204.7		192.4	194.4	195.3	195.7	196.0
HT30350AP					197.5	197.8	198.1	198.2	198.3
HT31080					165.9	166.7	167.0	167.1	167.2
HT31100APS					166.4	168.4	169.5	169.9	170.3
HT31140AP					178.2	178.9	179.1	179.2	179.2
HT31150APS					178.7	180.4	181.3	181.6	182.0
HT32080					167.7	169.5	170.2	170.5	170.7
HT32100S					167.7	170.5	171.6	171.9	172.2
HT32110W					169.0	174.1	175.0	175.2	175.5
HT32120					188.1	190.5	191.7	192.0	192.2
HT32140S	River Street	Collector	193.6	Υ	188.2	192.2	193.1	193.2	193.4
HT32145W					189.5	194.0	194.6	194.7	194.8
HT32150				Υ	189.7	193.9	194.8	194.9	194.9
HT32170S	Dow Street	Local	194.2	Υ	189.8	194.7	195.0	195.1	195.2
HT32180				Υ	189.9	194.7	195.1	195.1	195.2
HT32200S	Business Parkway	Local	193.8	Υ	189.9	194.7	195.1	195.1	195.2
HT33080					170.0	172.0	172.1	172.2	172.2
HT33100S	Norman Drive	Arterial	181.3		172.2	174.8	176.8	177.6	178.3
HT33100S2	Norman Drive	Arterial			170.3	178.0	177.7	177.7	182.5
HT33150AP					184.9	186.2	186.8	187.0	187.1
HT33190AP					191.5	192.0	192.2	192.2	192.3
HT33200APS					192.6	193.5	194.6	195.7	197.0
HT33290AP					192.3	193.0	193.3	193.4	193.5
HT33300APS					193.4	200.8	202.3	202.6	202.8

Notes:

- 1. Roads not meeting the City's defined Level of Service.
- 2. Roads not meeting the City's define Level of Service due to Withlacoochee flooding.
- 3. Water surface elevations due to Withlacoochee Staging.
- 3. 'Y' depicts potential structure flooding near the corresponding node location.
- 4. Potential Stucture flooding estimated by comparing model results with the regional 2 foot contours dataset. Additional finished floor elevations data shall be acquired for further investigation.
- 5. Potential Stucture flooding estimated by comparing model results with the regional 2 foot contours dataset. Additional finished floor elevations data should be acquired for further investigation.
- 6. All design storm events are 24 hour duration.

Yearly TSS loads were calculated based on standard EMC of TSS, yearly rainfall, tributary area; land use characteristics like percent imperviousness for Valdosta. Yearly TSS loads from various hydrologic units for each sub-basin were computed in lbs/year units. The total TSS loading of Hightower Creek was estimated to be 170,000 lbs/year.

The Georgia Stormwater Manual states the sizing criteria for any stormwater control/mitigation system to treat the runoff from 85 percent of the storms that occur in an average year. For Georgia, this equates to providing water quality treatment for the runoff resulting from a rainfall depth of 1.2 inches. This runoff is also termed as the Water Quality treatment volume (WQ $_{\rm V}$). Please refer to Georgia Stormwater Manual Volume 2 (technical handbook) Section 1.3 for a detailed discussion on WQ $_{\rm V}$ and the unified stormwater sizing criteria. The 1.2-inch storm event was simulated alternative projects evaluation.

Channel Bank Erosion: Almost 7,000 linear feet of Hightower Creek show velocities greater than 3 ft/sec. The threshold velocity for erosive velocity in Hightower Creek sub-basin is 3 ft/sec. Several locations were verified in field and showed signs of channel bank erosion.

4.3.5.3 Level of Service Summary

Under the present land use conditions, the 1.2-in, 5-, 25-, 50-, and 100-year 24-hour design storms were simulated to determine the problem areas as defined below.

In the Hightower Creek sub-basin the following roads do not meet the City's Level of Service as described in Section 2. The Hightower Creek stage Table 4.3.4 highlights all roads not meeting the level of service in red. For a road to be classified as not meeting the level of service, it has more than 6 inches of flooding for the storm event under consideration for that particular road classification (5-year event for a local road and 50-year event for a collector and arterial road).

Roads

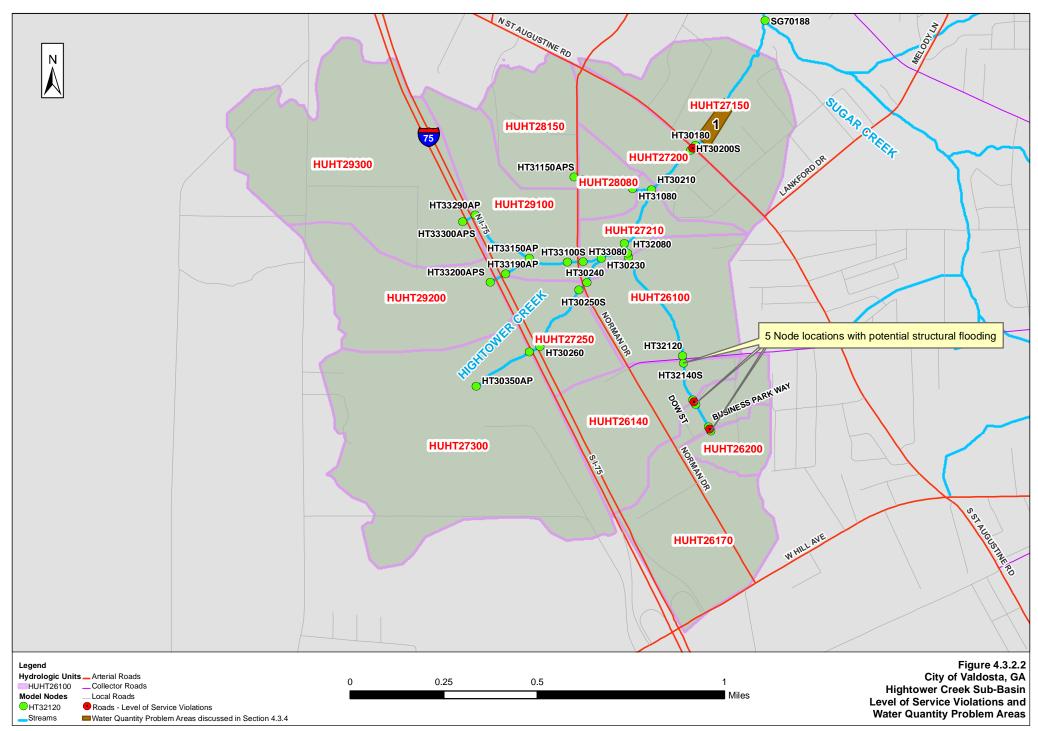
Two local roads (Dow Street and Business Parkway) do not meet the defined level of service (more than 6 inches of flooding for a 5-year storm event). One arterial road (St. Augustine Road) also does not meet the defined level of service (more than 6 inches of flooding for a 50-year storm event).

Structures

Five locations, as represented by model nodes, were identified in the Hightower Creek sub-basin for potential structural flooding for the 100-year event.

Please refer to **Figure 4.3.2.2** for a map of Level of Service violations for Hightower Creek sub-basin. Other water quantity problem areas are also shown on this figure.





4.3.6 Alternatives Evaluation

This section describes the alternatives evaluated for the Hightower Creek Sub-basin. Based on the screening process for the alternatives evaluation, the following alternatives representing different levels of service were developed. Detailed public safety options and standards should be considered and used during final design.

- Alternative HT1: RSF near Hightower Circle
- Alternative HT2: Grade Control structure downstream of St. Augustine Road

Alternative HT1 - RSF near Hightower Circle

This Alternative consists of constructing a 3.5-acre Regional Stormwater Facility (RSF) near Hightower Circle. The proposed RSF has a Permanent Pool Volume (Water Quality Volume) of 9.0 ac-ft and a Residence Time of 4.8 days. The tributary area to the facility is 793 acres. The implementation of this alternative brings arterial road (St. Augustine Road) into Level of Service compliance along with reducing sediment loads in Sugar Creek.

The proposed RSF will treat 793 acres of untreated tributary area, and therefore will reduce sediment load. Land acquisition would be required. The location of the proposed RSF is shown on **Figure 4.3.3**. **Table 4.3.5** shows the conceptual cost estimates for this alternative.

Alternative HT2- Grade Control Structure Downstream of St. Augustine Road

Alternative BR2 includes construction of a grade control structure just downstream of the N. St. Augustine Road. During field investigation erosion was observed in this section, hence a designed series of engineered drop structures is proposed at this location on Browns Canal as shown on **Figure 4.3.4**. In addition to providing reduction in channel erosion and reduction in high velocities in the stream, this project is part of a basin-wide effort to stabilize and restore the stream. **Table 4.3.6** shows the conceptual cost estimates for this alternative. CDM recommends detailed geomorphologic assessment is carried out before commencing any design or construction for a grade control project.



